

ME201 Project 2: Deformation

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1.1 Deformation Gradients

The deformation gradient, is defined by $\mathbf{F}(\mathbf{t}) = \nabla x = \frac{dx}{dX}$ where:

$$x = u + X \quad (1)$$

. To define the deformation gradient in terms of $A(t)$, we can write x as $u + X$, and take the gradient w.r.t X yielding: $\mathbf{F}(\mathbf{t}) = \nabla x = \nabla u + I$. We know that u w.r.t X is simply $A(t)$, so $\mathbf{F}(\mathbf{t}) = A(t) + I$. This correlates to a 3 x 3 matrix.

For $A(t)$ to be physically permissible, we need the following to hold:

$$0 < \det(\mathbf{F}(\mathbf{t})) < \infty \quad (2)$$

where $J = \det(\mathbf{F}(t))$. This is the case, because for deformation to occur, the deformation gradient must fall within all real-positive values. Values less than one will correspond to a shrink while values that tend towards infinity will indicate expansion. If there is no deformation at $t=0$ (undeformed), then $A(t) = 0$ and $J = \det(\mathbf{F}(t)) = \det(A(t) + I) = 1$. This relation can be observed by the following relation of conservation of mass:

$$\frac{m}{dV} = \frac{mJ}{dV_0} \quad (3)$$

$$\rho_{final} = J\rho_0 = \rho_0 \quad (4)$$

The expression for the deformed state of a matrix \mathbf{A} can be written in terms of ρ_0 ρ given the relation (2). Rearranging:

$$\rho_0 = \rho(\det(\mathbf{A}(t) + I)) \quad (5)$$

where ρ_0 and ρ are the original and final densities, respectively. As aforementioned, the relation \mathbf{A} must satisfy for volume preservation is that the Jacobian (the determinant of the deformation matrix $\mathbf{F}(t)$) must be equal to one.

1.2 Determining Components of Deformation

This section considers the laws relating the second Piola-Kirchhoff stress to the Green-Lagrange strain with bulk and shear moduli κ and μ (see appendix)

A second order Green-Lagrange Strain, E can be determined via using the provided $A(t)$ matrix:

$$E = \frac{1}{2} * F^T * F - I \quad (6)$$

$$F(t) = \begin{bmatrix} 2 & 0 & 0 \\ 0 & 2 & 1 \\ 0 & 0 & 2 \end{bmatrix} \quad (7)$$

$$E = \begin{bmatrix} 0 & 0 & 0 \\ 0 & 0 & 0.5 \\ 0 & 0.5 & 0.5 \end{bmatrix} \quad (8)$$

The Second-Order Piola-Kirchhoff Stress can be calculated using the relations in the appendix assuming isotropic material behavior.

$$S = E_L * ES = \begin{bmatrix} \frac{k}{2} - \frac{\mu}{3} \\ \frac{k}{2} - \frac{\mu}{3} \\ \frac{k}{2} + \frac{2\mu}{3} \\ 0 \\ \mu \\ 0 \end{bmatrix} \quad (9)$$

The Cauchy-Stress is calculated using:

$$\sigma = \frac{1}{J} * F * S * F^T \quad (10)$$

where $J = \det(F) = 8$

$$\sigma = \begin{bmatrix} \frac{k}{4} - \frac{\mu}{6} & 0 & 0 \\ 0 & \frac{5k}{16} + \frac{5\mu}{12} & \frac{k}{8} + \frac{2\mu}{3} \\ 0 & \frac{k}{8} + \frac{2\mu}{3} & \frac{k}{4} + \frac{\mu}{3} \end{bmatrix} \quad (11)$$

The volumetric dialation is the hydrostatic pressure constant determined from trace of the Cauchy-Stress. This will be used to evaluate the deviatoric stress, which is the difference between the Cauchy-Stress and the Hydrostatic Pressure calculated to determine the change of shape of a body.

$$P = \frac{\text{trace}(\sigma)}{3} = \frac{12k}{48} + \frac{7\mu}{36} \quad (12)$$

$$\sigma_{dev} = \begin{bmatrix} \frac{-k}{48} - \frac{13\mu}{36} & 0 & 0 \\ 0 & \frac{k}{24} + \frac{2\mu}{9} & \frac{k}{8} + \frac{2\mu}{3} \\ 0 & \frac{k}{8} + \frac{2\mu}{3} & \frac{-k}{48} + \frac{5\mu}{36} \end{bmatrix} \quad (13)$$

1.3 Arbitrary Uniform Displacement with Isotropic Material Properties

If the initial location of a material point is given by $X = [1 \ 1 \ 1]^T$ then the deformed location of \mathbf{x} (at the same material point) is determined by:

$u = A(t) * X$ and using eq.(1)

$$x = \begin{bmatrix} 1.02 \\ 1.05 \\ 1.06 \end{bmatrix} \quad (14)$$

If, on the other hand we are given the deformed location of a material point instead, x , then the inverse is true. Obeying the linear algebra:

$X = (A(t) + I)^{-1} * x$

$$X = \begin{bmatrix} 0.9810 \\ 0.9523 \\ 0.9429 \end{bmatrix} \quad (15)$$

The Cauchy-Stress Tensor for the deformation discussed in (4) and using eq.(10) is the following:

$$\sigma = \frac{1}{1.05} \begin{bmatrix} 1 & 0.01 & 0.01 \\ 0.01 & 1.02 & 0.02 \\ 0.01 & 0.02 & 1.03 \end{bmatrix} * \begin{bmatrix} s_{11} & s_{12} & s_{13} \\ s_{21} & s_{22} & s_{23} \\ s_{31} & s_{32} & s_{33} \end{bmatrix} * \begin{bmatrix} 1 & 0.01 & 0.01 \\ 0.01 & 1.02 & 0.02 \\ 0.01 & 0.02 & 1.03 \end{bmatrix} \quad (16)$$

To determine a traction vector, we use the following:

$$t = \sigma * n \quad (17)$$

With knowledge of the surface, we can determine the gradient of the surface, which is *ALWAYS* normal to the surface. This will help in determining n . The gradient of the function provided is: $\nabla s = 2e^{(2x_1)} + 2x_2 + 2\sin(2x_3)$ with magnitude $||\nabla s|| = \sqrt{4e^{(4x_1)} + 4\sin(2x_3)^2 + 4x_2^2}$. $n = \frac{\nabla s}{||\nabla s||}$. The calculations for the traction vector can be found in the appendix

1.4 Reference Configuration of Deformation using Balance of Linear Momentum

$$\nabla_x E(t) * S(t) + f = \rho \ddot{x} = \nabla_x \sigma(t) + f = \rho \ddot{x}$$

$$\nabla_x \sigma(t) = 0$$

$$x = u + X \quad \dot{x} = \dot{u} + \dot{X} = \dot{u} \quad \ddot{x} = \ddot{u}$$

$$U = A(t) * X \implies \ddot{u} = \ddot{A}(t) * X = \ddot{x}$$

$$f = \rho \ddot{x} \implies \rho(\ddot{A}(t) * X)$$

Converting to Referential Coordinates

$$\rho = \rho_0 J = \rho_0 (\det(A(t) + I))$$

$$f = \frac{\ddot{A}(t) * X}{\det(A(t) + I)}$$

1.5 Determining λ Principle Stresses and Directions

The eigen-value problem is first determined by *eq:(10)* and solving

$$\sigma v = \lambda v \implies (\sigma - \lambda I) = 0 \quad (18)$$

$$\sigma = \begin{bmatrix} d & 0 & d \\ 0 & \frac{1}{9} & 0 \\ d & 0 & -d \end{bmatrix}$$

To solve for Eigen-Values, we subtract a diagonal lambda matrix from σ and take the determinant. This can also be done in MatLab, using the eig(:) command. The Eigen-Values, which correspond to the **principal stresses** for the C-S Tensor are:

$$\lambda = \begin{bmatrix} \frac{1}{9} \\ d\sqrt{2} \\ -d\sqrt{2} \end{bmatrix}$$

The Eigen-Vectors, which correspond to the **principal stress directions** for the C-S Tensor are calculated by taking the maximum λ value and solving for v in *eq:(18)*. The Eigen-Vectors can also be determined in Matlab using the eig(:) command.

$$v = \begin{bmatrix} 0 & \sqrt{2} + 1 & 1 - \sqrt{2} \\ 1 & 0 & 0 \\ 0 & 1 & 1 \end{bmatrix}$$

Here, we know that the principal stress of largest magnitude is $\lambda = d\sqrt{2}$ which corresponds to the direction of $[x_1 \ x_2 \ x_3] = [\sqrt{2} + 1 \ 0 \ 1]$.

1.6 Von Mises Failure Criterion

$$\sigma_v^2 = \frac{1}{2} [(\sigma_{11} - \sigma_{22})^2 + (\sigma_{22} - \sigma_{33})^2 + (\sigma_{33} - \sigma_{11})^2 + 6(\sigma_{23}^2 + \sigma_{31}^2 + \sigma_{12}^2)]$$

$$\sigma_Y = \sqrt{\frac{d - \frac{1}{9}}{2} + \frac{d + \frac{1}{9}}{2} + 5d^2}$$

and rewriting into terms of known constant σ_Y we get

$$d = \sqrt{\frac{81\sigma_Y^2 - 1}{9\sqrt{6}}}$$

which is the value of d at which yielding will occur using the Von Mises Stress Criterion

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Basics of Deformation Problem 2,3

Below, we compute deformation parameters assuming an arbitrary starting condition

```
F = [1 0 0;0 1 1;0 0 1]+eye(3);
A_prime = [1 0 0;0 1 1;0 0 1];
E = 0.5*(A_prime'*A_prime-eye(3));
E6=[E(1,1);E(2,2);E(3,3);2*E(1,2);2*E(2,3);2*E(3,1)];% E-values
provided
syms k
syms mu
c1=k+4/3*mu; % constant k
c2 = k-2/3*mu; % constant
EE=[c1,c2,c2;c2,c1,c2;c2,c2,c1] % Green Lagrange
imu = eye(3)*mu; %mu's
EE=[EE zeros(3,3);zeros(3,3) imu]; % Green lagrange padded with zeros
S =EE*E6 % second piola-kirchoff
S_3 = [S(1) 0 0; 0 S(2) mu ; 0 mu S(3)];
sigma = 1/det(F)*(F*S_3*F')%cauchy stress
P = diag([1/3 * trace(sigma) 1/3 * trace(sigma) 1/3 * trace(sigma)]) %
Pressure Tensor (volumetric component of sigma)
dev_sigma = sigma - P %deviatoric stress
```

$EE =$

```
[ k + (4*mu)/3, k - (2*mu)/3, k - (2*mu)/3]
[ k - (2*mu)/3, k + (4*mu)/3, k - (2*mu)/3]
[ k - (2*mu)/3, k - (2*mu)/3, k + (4*mu)/3]
```

```
S =      k/2 - mu/3
      k/2 - mu/3
      k/2 + (2*mu)/3
              0
              mu
              0
```

$\sigma =$

$$\begin{bmatrix} k/4 - \mu/6, & 0, & 0 \\ 0, & (5*k)/16 + (5*\mu)/12, & k/8 + (2*\mu)/3 \\ 0, & k/8 + (2*\mu)/3, & k/4 + \mu/3 \end{bmatrix}$$

$P =$

$$\begin{bmatrix} (13*k)/48 + (7*\mu)/36, & 0, & 0 \\ 0, & (13*k)/48 + (7*\mu)/36, & 0 \\ 0, & 0, & (13*k)/48 + (7*\mu)/36 \end{bmatrix}$$

$dev_sigma =$

$$\begin{bmatrix} -k/48 - (13*\mu)/36, & 0, & 0 \\ 0, & k/24 + (2*\mu)/9, & k/8 + (2*\mu)/3 \\ 0, & k/8 + (2*\mu)/3, & (5*\mu)/36 - k/48 \end{bmatrix}$$

Consideraton of a different displacement

```
A = 0.01 * [0 1 1; 1 2 2; 1 2 3];
%Part A
X_init = [1 1 1]';
g=A+eye(3);
u=A*X_init;
little_x = u+X_init
% Part B
x_final = [1 1 1]';
X_prev=inv(g)*x_final

% Part C
syms s11 s22 s33 s12 s13 s23 s31 s32 s21
J=det(F)
S=[s11 s21 s31; s12 s22 s13; s13 s23 s33];
sigma_cauchy = 1/J * ( F * S * F') % write in terms of a product of
matrices

% part D
syms x1 x2 x3
s = exp(2*x1)+ x2^2 - cos(2*x3);
g_s=gradient(s);
g_n=norm(g_s);
n=g_s/g_n
t= sigma_cauchy*n %traction vector
```

little_x =

1.0200
1.0500
1.0600

X_prev =

0.9810
0.9523
0.9429

J =

8

sigma_cauchy =

[s11/2, s21/2 + s31/4, s31/2]
[s12/2 + s13/4, s13/4 + s22/2 + s23/4 + s33/8, s13/2 + s33/4]
[s13/2, s23/2 + s33/4, s33/2]

n =

(2*exp(2*x1))/(4*abs(sin(2*x3))^2 + 4*abs(x2)^2 + 4*exp(4*real(x1)))^(1/2)
(2*x2)/(4*abs(sin(2*x3))^2 + 4*abs(x2)^2 + 4*exp(4*real(x1)))^(1/2)
(2*sin(2*x3))/(4*abs(sin(2*x3))^2 + 4*abs(x2)^2 + 4*exp(4*real(x1)))^(1/2)

NOTE: traction vector is poorly evaluated in MatLab

t =

(2*x2*(s21/2 + s31/4))/(4*abs(sin(2*x3))^2 + 4*abs(x2)^2 + 4*exp(4*real(x1)))^(1/2)
+ (s11*exp(2*x1))/(4*abs(sin(2*x3))^2 + 4*abs(x2)^2 + 4*exp(4*real(x1)))^(1/2) + (s31*sin(2*x3))/(4*abs(sin(2*x3))^2 + 4*abs(x2)^2 + 4*exp(4*real(x1)))^(1/2)
(2*x2*(s13/4 + s22/2 + s23/4 + s33/8))/(4*abs(sin(2*x3))^2 + 4*abs(x2)^2 + 4*exp(4*real(x1)))^(1/2) + (2*exp(2*x1)*(s12/2 + s13/4))/(4*abs(sin(2*x3))^2 + 4*abs(x2)^2 + 4*exp(4*real(x1)))^(1/2)
+ (2*sin(2*x3)*(s13/2 + s33/4))/(4*abs(sin(2*x3))^2 + 4*abs(x2)^2 + 4*exp(4*real(x1)))^(1/2)
(2*x2*(s23/2 + s33/4))/(4*abs(sin(2*x3))^2 + 4*abs(x2)^2 + 4*exp(4*real(x1)))^(1/2)
+ (s13*exp(2*x1))/(4*abs(sin(2*x3))^2 + 4*abs(x2)^2 + 4*exp(4*real(x1)))^(1/2) + (s33*sin(2*x3))/(4*abs(sin(2*x3))^2 + 4*abs(x2)^2 + 4*exp(4*real(x1)))^(1/2)

~~mu~~
0

New Material Deformations, Principal Cauchy Stress

```
syms d
F= [3 0 0; 0 1 0; 0 0 3];
S = [d 0 d; 0 1 0; d 0 -d];
J=det(F)
sigma = 1/J *( F * S * F')
[vectors, values] = eig(sigma)
direction = vectors(:,2)

% determine the von mises criterion
sigma_y = (0.5*((sigma(1,1) - sigma(2,2))^2 + (sigma(2,2) -
sigma(3,3))^2 + (sigma(3,3)-sigma(1,1))^2 + 6*(sigma(2,3)^2 +
sigma(1,3)^2 + sigma(1,2)^2)))^(1/2)

J =

     9

sigma =

[ d, 0, d]
[ 0, 1/9, 0]
[ d, 0, -d]

vectors =

[ 0, 2^(1/2) + 1, 1 - 2^(1/2)]
[ 1, 0, 0]
[ 0, 1, 1]

values =

[ 1/9, 0, 0]
[ 0, 2^(1/2)*d, 0]
[ 0, 0, -2^(1/2)*d]

direction =      sigma_y =

2^(1/2) + 1      ((d - 1/9)^2/2 + (d + 1/9)^2/2 + 5*d^2)^(1/2)
0
1
```